Allowable Stress Design of

SIMPLE SHEAR CONNECTIONS

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Allowable Streb Design Of Simple Shear Connections:

Allowable Stress Design of Simple Shear Connections American Institute of Steel Construction,1990 **Theoretical Chemical Engineering Abstracts** ,1986 *Physics Briefs* ,1993 Simple Shear Connections American Institute of Steel Construction (New York, N.Y.),1990 *Load and Resistance Factor Design of Simple Shear Connections* Dorothy Brown Thompson,1990-09-01 Load and Resistance Factor Design of Simple Shear Connections American Institute of Steel Construction,1990 *Testing of Extended Shear Tab Connections Subjected to Shear* Jacob Hertz,2014 Shear tab or single plate connections are widely used as simple shear connections in the construction of steel structures These connections take the form of a single plate shop welded to a supporting column or girder During erection the supported beam is moved into place and connected to the shear tab using bolts In some cases the eccentricity of the bolt group to the face of the supporting member must be increased due to congestion near the support face or for constructability In this case the shear tab connection is considered extended the alternative being conventional The Canadian Institute of Steel Construction CISC Handbook of Steel Construction and the American Institute for Steel Construction AISC Steel Construction Manual both include pre designed conventional shear tab connections the shear resistances of which were computed using the AISC design method confirmed through testing by Astaneh et al 1989 In addition the AISC Manual includes a design method for extended configurations This research aims to verify the accuracy in predicting the shear resistance of extended shear tab connections using a modified method combining that of CSA S16 09 2009 the CISC Handbook 2010 and the AISC Manual 2010 The shear resistances of 12 representative shear tab connections were predicted using said method and compared with the measured resistances found through full scale testing Four beam to column and eight beam to girder extended shear tab connections were tested in the Macdonald Engineering Jamieson Structures Laboratory at McGill University Two of the four beam to column tests were governed by flexural tearing of the weld The welds were sized as specified in the AISC design method at 5 8ths of the plate thickness which assumes 345MPa steel welded with E49 electrodes This author recommends the welds be sized using a design equation that takes into account the probable yield stress of the steel The other two beam to column tests resulted in plastic local buckling of the bottom edge of the shear tab The AISC design method allows for the buckling resistance to be calculated using two models i lateral torsional buckling or ii a conservative classical plate buckling The measured buckling resistances for both tests were significantly better predicted by the latter model The beam to girder tests revealed that two limit states should be accounted for in the design method i biaxial buckling of full height connections and ii localized deformation of the supporting girder web and flange for partial height connections Design equations are proposed for both of these limit states *A Comparison of Double Clip Angle Shear Connections to Shear Tab Connections in Industrial Applications* Brandi Nichole Martin,2017 In structural steel connection design simple shear connections are one of the most common connection types utilized The industry especially from the side of the engineer tends to lean toward

using Double Clip Angle Connections as the default standard for simple shear connections A double clip angle connection is a connection consisting of two angles transferring the shear forces from one member to the next either through bolts or welds The design of Double Clip Angle Connections is efficient and the connections themselves are easy to fabricate However benefits to utilizing other types of shear connections exist Many of these benefits are seen in the fabrication shop or during erection and construction This is especially true of single shear plate or shear tab connections when applied to open structure design Shear tab connections consist of a single plate that transfers the shear forces from one member to the next with bolts or with welds The design of shear tab connections can be a more involved process than the design of double clip angles Sometimes the shear plate or shear tab has to be longer than is typical This is called an extended shear plate connection These extended shear plates can bring other variables into the design that typically don t occur with Double Clip Angle Connections such as bending of the plate or the need for multiple bolt columns However with proper planning and detailing the benefits and savings experienced in the fabrication or construction phase may outweigh what can be seen as a more laborious design task The purpose of this report is to identify the possible benefits achieved in using each of these connections highlight the differences in the design approach for each and use a study model to compare the outcome of using one connection over another in the design of a typical open structure Double clip angles are typically the most efficient approach when speed of design and simplicity of fabrication are the desired outcomes However shear plate or shear tab connections have the potential to provide safer erection alternatives and materials savings if used in appropriate ways and with the right applications Strength and Ductility Requirements for Simple Shear Connections with Shear and Axial Load William A. Thornton,1997* **Non-linear Finite Element Analysis of Extended Shear Tab Connections** Mohamed Fawzi Suleiman,2013 The Manual of Steel Construction AISC 14th edition refers to an extended shear tab as a single plate shear connection This method of providing simple connections has become quite popular with both fabricators and erectors Extended shear tab connections were formally introduced in the 13th edition of AISC Steel Construction Manual Using experimental data from extended shear tab connections Sherman and Ghorbanpoor introduced a design methodology in 2002 for extended shear connections Twisting of the shear tab controlled the capacity of the specimens tested by Sherman and Ghorbanpoor which were not laterally braced In the latest edition of AISC Steel Manual design equations are provided to assess the need for stabilizer plates in the connection region of extended shear tabs In an effort to understand whether twisting of the shear tab can be a controlling design limit state three dimensional nonlinear finite element analyses in conjunction with design case studies were conducted The analyses included 364 connections with different configurations were studided with an a distance of 9 11 and 16 inches The finite element models were comprehensive in terms of simulating nonlinear material properties boundary conditions pretensioning in the bolts geometric nonlinearity etc It was possible to accurately replicate the responses shear force connection vertical deflection and shear force connection angle of twist

measured in a number of previous tests and to fairly well predict the observed failure modes Using a 3D nonlinear finite element analysis technique the response of 16 selected connections which had been designed to meet all the applicable limit states in AISC Steel Manual were evaluated The presence of floor slab which braces the top flange of the beam was simulated in the analyses For a number of cases the connection behavior at the ultimate limit state was dominated by twisting i e the relationship between torsional moment and angle twist indicated a noticeable level of loss of stiffness in comparison to that from the shear vertical displacement relationship However the level of lateral displacement of the shear tab was small particularly for unfactored loads when control of deformations is an important design objective According to AISC provisions Eq 10 6 stabilizer plates would not be required for any of these 16 connections which were evaluated by 3D nonlinear FEA Therefore current AISC provisions are a good predictor of the expected level of out of plane displacement of the shear tab due to twisting It should be noted that large lateral displacements occurred at the ultimate state when the connection ductility is the main design consideration but not the magnitudes of deformations and distortions Therfore this equation can be used to determine whether stabilizer plates are needed or not however it does not predict whether the response at the ultimate limit state will be dominated by excessive loss of torsional stiffness of the shear tab Instead of using stabilizer plates a thicker plate can be used for the shear tab This solution is considered to be more economical and easier than welding stabilizer plates in the connection region **Behavior of Single Plate Shear Connections with Rigid and Flexible Supports** ,2004 Single plate framing connections are a type of shear connection which must sustain both shear and moment transferred from the supported beam Commonly found in both rigid beam to column flange and flexible beam to girder web configurations the single plate connection has proven to be both economical and easy to erect A specified design method for single plate connections is presented in the AISC LRFD 3rd Edition Manual AISC 2003 The design method calculates for associated limit states or failure modes and accounts for eccentric shear created by the moment sustained by the connection Overly conservative tabulated design strengths which lead to heavier more costly connections has emphasized the need for research to improve upon the current design procedure The focus of this research is to address the conservative nature of the currently specified design method To create a basis for comparison to theoretically calculated design strengths a total of ten full scale tests were conducted Tests incorporated both rigid and flexible support conditions both standard and short slotted bolt holes and connections consisting of various numbers of bolts Differing from previous research efforts this series of research incorporated a simulated slab restraint with flexible supported tests In order to evaluate experimental results and remove unknowns supplemental tests were conducted to determine the material properties of components used in testing Major goals accomplished in this research included the investigation of connection limit states and component behavior Focus was given to qualifying rotational behavior and quantifying the extent that eccentric shear is experienced by the connection as this is felt to be a potential cause of the overly conservative tabulated design values In addition

comparisons of the AISC design method were made to design methods proposed by other researchers and design methods specified in other countries All investigations f **Ultimate Tensile Deformation and Strength Capacity of Shear Tab Connections** Chen Zhang,2019 Single plate or shear tab is a common simple connection to connect steel beams to columns The connection is traditionally designed for the shear load transferred from the supported beam only while it has long been recognized that the shear connection can resist a certain amount of tensile force in the longitudinal direction of the supported beam which is critical to preserve the integrity of a structure Canadian standard CSA S16 14 explicitly states that connections shall be designed to provide resistance to progressive collapse as a consequence of a local failure However few specific design requirements are provided in the standard Hence the main objective of this research is to quantify the deformation and strength capacities of shear tab connections when subjected to a pure tension or a combined tension and shear load in the context of progressive collapse resistance First a set of full scale shear tab connection specimens were tested under a pure tension load The results from the experiments are then used to verify and calibrate a finite element model of the connections Thirdly the finite element model is used to conduct a parametric study to determine the impact of tab thickness tab edge distance bolt diameter and the combined effect of tension and shear load Finally a formulation describing the relationship between the tensile force and the axial deformation for the shear tab connections is developed

One-sided Steel Shear Connections in Column Removal Scenario Hossein Daneshvar,University of Alberta. Department of Civil and Environmental Engineering,2014 There are many design methodologies and philosophies intended to provide structural integrity or increase structural robustness thereby making structures resistant to progressive collapse However there is little information that reveals sources and levels of inherent robustness in structural steel members and systems The present study seeks to begin the process of behaviour evaluation of components and assemblages initially designed for other purposes than progressive collapse such as gravity loads and make recommendations regarding their performance and possible methods for improvements for the new scenario These recommendations can lead to more economical design and safer structural steel systems in the event of localised damage that has the potential to spread to a disproportionately large part of the structure Connections play a major role in ensuring general integrity of different types of steel structural systems Hence numerical investigations have been performed to extend the current body of knowledge on connections and consequently the structural response in the event of progressive collapse This study is intended to examine the response of steel frames with simple shear connections in the aftermath of unusual and extreme localized loads The main goal of this research is to evaluate the behaviour of some prevalent and economical one sided i e connected only on one side of the supported beam web shear connection types shear tab tee WT and single angle in buildings and perform numerical analyses on those connection configurations under extreme loading scenarios represented generically by the socalled column removal scenario Characteristic features of the connection response such as the potential to develop a reliable alternative

path load through catenary action and ultimate rotational capacities are discussed to provide a solid foundation for assessing the performance of buildings with these types of connections Observations regarding the analysis results are synthesized and conclusions are drawn with respect to the demands placed on the connections The results of this research project should contribute to a better understanding of the resistance of steel structures with one sided shear connections to progressive collapse **Structural Shear Joints** G T Hahn,C A Rubin,K A Iyer,2005-12-06 Engineers employ shear connections riveted and bolted butt and lap joints in a wide range of structures and machines Ordinary stress analysis of joints are unable to define the features that ultimately govern fatigue and fretting and provide bases for design Detailed analysis has only become possible recently with advances in finite element methods and computing capabilities This text places into context the results of over 150 detailed 2D and 3D finite element analyses of aluminum and steel shear connections so that engineers can optimise the design and reliability of shear joints Understand how to optimise the design and reliability of shear joints Places into context the results of over 150 detailed 2D and 3D finite element analysis of aluminium and steel sheer connections Beam-to-column Web Shear Tab Connections – Minor Axis Column Loading Andrea Iachetta,2019 Single plate shear tab connections are commonly used due to their relatively simple design ease of fabrication and safe erection on site They are fillet welded to a supporting column or girder and then bolted to a supported beam Extended shear tabs can either frame into the web a flexible support or the flange a rigid support of a column The shear tab connection is assumed to function as a pin in the analysis of the overall structure However testing has shown that it must withstand shear and moment associated with the eccentricity applied to the shear tab relative to the support Current design procedures have little guidance on how to account for this added moment to the column which may not be a concern for shear tab connections framing into a rigid support column bending about major axis but may be more critical when framing into a column s minor axis which is considered as a flexible support condition The existing design procedures for shear tab connections were formulated from past research studies The 2015 CISC Handbook of Steel Construction lacks a design approach for extended shear tabs The 2017 AISC Steel Construction Manual has a conservative design approach only based on unstiffened connections despite the frequent use of stiffened extended shear tab connections Further research and testing is required to provide recommendations for a more efficient and complete design procedure Finite element models were used to analyse the behaviour and capacity of beam to column web extended shear tab connections Five validation models were first created to simulate laboratory tests by D Aronco 2013 to verify the modelling assumptions The obtained shear forces rotations and displacements proved to be accurate confirming that the modelling approach could be used in a subsequent parametric study The parametric study included the influence of the supporting column single vs double sided connections the effect of the type and connection pattern of stabilizer plates the addition of vertical rows of bolts the shape of the shear plate and the effect of the loading protocol The results demonstrate that the column size the addition of a vertical row of bolts and the

shape of the shear tab did not alter the connection capacity Also the welds connecting the stabilizer plates to the column web are not necessary if the plates are welded to the column flanges Introducing a gap between the stabilizer plates and the column flanges can benefit the column by reducing the moments applied to it Finally the loading protocol greatly influences the connection behaviour more studies are required to further investigate this aspect *Design of Shear Tab Connections for Gravity and Seismic Loads* Abolhassan Astaneh-Asl,Steel Committee of California,2005 **Contribution of Shear Connections to the Lateral Stiffness and Strength of Steel Frames** Melinda Barber,2011 Spurred by the abundance of welded moment connection failures observed in the Northridge earthquake of 1994 research efforts involving the capacity of simple gravity connections have since increased significantly Typically a building includes more gravity connections than moment connections yet the moment resistance contributions of the gravity connections are neglected in design Based on the lack of complete building collapse resulting from the Northridge earthquake this paper analytically investigates the overall lateral stiffness and strength contributions of the system s shear connections This paper first investigates the differences between fully restrained FR partially restrained PR and simple shear connections Next previous experimental test setups and results are examined Finally ETABS finite element modeling techniques including an elastic hinge analysis and pushover analyses using both default and user defined hinge parameters are compared PR hinge properties for three experimental gravity connections will be explicitly defined To allow application to a broader spectrum of gravity beam sizes equations to define the PR hinge will be developed using properties of the beam Ultimately a prototype building will be modeled in ETABS for two different connection layouts Once using the traditional shear connections and again by replacing the shear connections with the PR connections previously defined by hinges Comparing lateral drifts of these two models will provide quantification of the shear connection s overall lateral stiffness contribution **Comparison of Load and Resistance Factor Design and Allowable Stress Design for Simple Shear Connections** Monika G. Kluger,1996 Investigation of Simple Shear Connection Economy John L. Reese,2008 **Design of Single Plate Shear Connection** Abolhassan Astaneh-Asl,Stephen M. Call,Kurt Michael McMullin,Ralph M. Richard,Steel Committee of California,1990

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